

Canal Design

Using Kennedy's (1895) theory

$$V_0 = c_1 \cdot y^{c_2}$$

$$V_0 = 0.55 m y^{0.64}$$

...(4.)

where V_0 = Critical velocity in the channel in m/s.

y = water depth in channel in m

$$m = C.V.R$$

Table 4.2. Recommended Values of C.V.R. (m)

<i>S.No.</i>	<i>Type of silt</i>	<i>Value of m</i>
1.	Silt of River Indus (Pakistan)	0.7
2.	Light sandy silt in North Indian Rivers	1.0
3.	Light sandy silt, a little coarser	1.1
4.	Sandy, loamy silt	1.2
5.	Debris of hard soil	1.3

Kutter's Formula

$$V = \frac{\frac{1}{n} + \left(23 + \frac{0.00155}{S}\right)}{\left[1 + \left(23 + \frac{0.00155}{S}\right) \frac{n}{\sqrt{R}}\right]} \sqrt{RS}$$

Manning's Formula

$$V = \frac{1}{n} R^{2/3} \cdot S^{1/2}$$

Chezy's Formula

$$V = C \sqrt{RS}$$

Example 4.6. Design an irrigation channel to carry 50 cumecs of discharge. The channel is to be laid at a slope of 1 in 4000. The critical velocity ratio for the soil is 1.1. Use Kutter's rugosity coefficient as 0.023.

Solution. $Q = 50$ cumecs, $S = \frac{1}{4000}$
 $m = 1.1,$ $n = 0.023$

Use equation (4.19), as, $V_0 = 0.55m \cdot y^{0.64}$

Assume a depth equal to 2 m

$$V_0 = 0.55 \times 1.1 \times (2)^{0.64} = 0.605 \times 1.558 = \mathbf{0.942 \text{ m/sec}}$$

$$A = \frac{Q}{V_0} = \frac{50}{0.942} = 53.1 \text{ m}^2.$$

Assume side slopes as $\frac{1}{2} : 1 \left(\frac{1}{2} H : 1 V \right)$

Now, $A = y \left(b + y \cdot \frac{1}{2} \right)$

$\therefore 53.1 = 2(b + 1)$

or

$26.55 = b + 1$

or

$b = 25.55 \text{ m}$

and

$P = b + 2 \sqrt{\left(1 + \frac{1}{4} \right)} \times y$

or

$P = b + 2 \frac{\sqrt{5}}{2} y = 25.55 + \sqrt{5} \times 2 = 30.03$

$R = \frac{A}{P} = \frac{53.1}{30.03} = 1.77 \text{ m.}$

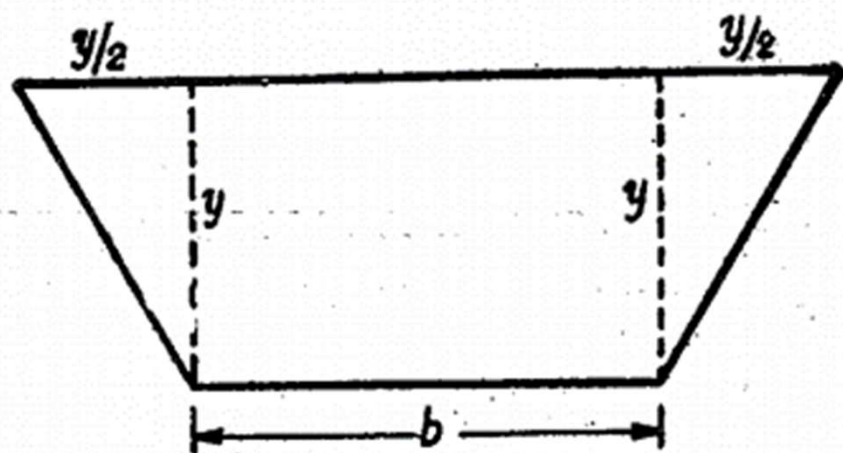


Fig. 4.9.

But, from eqn. (4.20),

$$\begin{aligned}
 V &= \left[\frac{\frac{1}{n} + \left(23 + \frac{0.00155}{S} \right)}{1 + \left(23 + \frac{0.00155}{S} \right) \frac{n}{\sqrt{R}}} \right] \sqrt{RS} \\
 \therefore V &= \left[\frac{\frac{1}{0.023} + 23 + \frac{0.00155}{1/4000}}{1 + \left(23 + \frac{0.00155}{1/4000} \right) \frac{0.023}{\sqrt{1.77}}} \right] \sqrt{1.77 \times \frac{1}{4000}} \\
 &= \left[\frac{43.5 + (23 + 6.2)}{1 + \frac{29.2 \times 0.023}{1.33}} \right] \left[1.33 \times \frac{1}{63.3} \right] \\
 &= \frac{72.7}{1 + 0.505} \times 1.33 \times \frac{1}{63.3} = \frac{72.7}{1.505} \times 1.33 \times \frac{1}{63.3} \\
 &= 1.016 \text{ m/sec} > 0.942; \text{ or } V > V_0.
 \end{aligned}$$

In order to *increase* the critical velocity (V_0), we have to *increase* the depth. So increase the depth.

Use 3 m depth :

$$V_0 = 0.605 \times (3)^{0.64} = 0.605 \times 2.02 = 1.22 \text{ m/sec.}$$

$$A = \frac{50}{1.22} = 40.8 \text{ m}^2.$$

$$40.8 = 3 \left(b + \frac{1}{2} \cdot 3 \right)$$

or $13.6 - 1.5 = b = 12.1 \text{ m.}$

$$P = 12.1 + 2 \times \frac{\sqrt{5}}{2} 3 = 12.1 + 6.72 = 18.82$$

$$R = \frac{A}{P} = \frac{40.8}{18.82} = 2.17 ; \text{ therefore } \sqrt{R} = 1.47.$$

$$V = \frac{43.5 + 29.2}{1 + \frac{29.2 \times 0.023}{1.47}} + \left[1.47 \times \frac{1}{63.3} \right] = \frac{72.7}{1.45} \times 1.47 \times \frac{1}{63.3}$$

$$= 1.16 \text{ m/sec.} < 1.22 ; \text{ or } V < V_0$$

So reduce the depth.

Use 2.5 m depth

$$V_0 = 0.605 \times (2.5)^{0.64} = 0.605 \times 1.797 = 1.087 \text{ m/sec.}$$

$$A = \frac{50}{1.087} = 46$$

$$46 = 2.5 (b + \frac{1}{2} \cdot 2.5)$$

$$18.4 - 1.25 = b = 17.15 \text{ m}$$

$$P = 17.15 + \sqrt{5} \times 2.5 = 17.15 + 5.58 = 22.73$$

$$R = \frac{A}{P} = \frac{4}{22.73} = 2.02 ; \text{ therefore } \sqrt{R} = 1.42$$

$$V = \frac{72.7}{1 + \frac{29.2 \times 0.023}{1.42}} (1.42) \left(\frac{1}{63.3} \right) = \frac{72.7}{1.472} \times \frac{1.42}{63.3}$$

$$= 1.1 \text{ m/sec} > 1.087 ; V > V_0$$

So increase the depth.

Use 2.7 m depth

$$V_0 = 0.605 \times 1.189 = 1.147$$

$$A = \frac{50}{1.147} = 43.5$$

$$43.5 = 2.8 \left(b + \frac{1}{2} \cdot 2.8 \right)$$

$$15.54 - 1.4 = b = 14.14 \text{ m}$$

$$P = 14.14 + \sqrt{5} \times 2.8 = 14.14 + 6.26 = 20.40$$

$$R = \frac{43.5}{20.4} = 2.13, \text{ therefore, } \sqrt{R} = 1.46$$

$$\begin{aligned} \therefore V &= \left[\frac{72.7}{1 + \frac{29.2 \times 0.023}{1.46}} \right] \left[\frac{1.46}{63.3} \right] = \left[\frac{72.6}{1.46} \right] \left[\frac{1.46}{68.3} \right] \\ &= 1.148 \text{ m/sec} \approx 1.147 \text{ or } V \approx V_0. \end{aligned}$$

Actual velocity V tallies with V_0 .

Hence, use the depth equal to 2.7 m and base width 14.14 m. (say 14.2 m) with slopes $\frac{1}{2} : 1$ of trapezoidal section. **Ans.**